

1. Identification of the product: **KEM HYBRID**

2. Identification code (art. 11.4), for the batch or serial number see packaging:

Type of Cartridge	Format	Cod.
Coaxial	150-280-300-330-380-410-420 ml	93600000000 /01-02-03-04-05-06-07-08 (420ml)

3. Intended use:

Generic type	Bonded anchor for anchorage of Threaded Rod and Rebar as ETA-16/0957
Base Material	Concrete C20/25 to C50/60 acc. to EN206-1
Use category	<ul style="list-style-type: none"> ▪ Installation in dry, wet concrete and flooded boreholes ▪ Overhead installation
Material & Durability	<ul style="list-style-type: none"> ▪ Threaded rod Galvanized cl.4.6 to cl.8.8 acc. to EN ISO898 for dry internal conditions ▪ Threaded rod Stainless Steel cl. A4-70 acc. to EN ISO3506 for internal and external use without particular aggressive conditions ▪ Threaded rod High Resistant Stainless Steel HCR-70 acc. to EN ISO3506 for all conditions ▪ Rebar Class B and C as EN 1992-1-1:2004+AC:2010, Annex C
Loading	Static, quasi-static and Seismic load
Temperature Range	- 40°C to +80°C max long term temperature +50°C and max short term temperature +80°C - 40°C to +120°C max long term temperature +72°C and max short term temperature +120°C - 40°C to +160°C max long term temperature +100°C and max short term temperature +160°C
Fire Reaction	A1 according to EN 13501-1

Generic type	Bonded anchor for anchorage of Post-Installed Rebar Connection as ETA-16/0961
Base material	Non-carbonated Concrete C12/15 to C50/60 acc. to EN206-1 [max 0,4 % CL]
Use category	<ul style="list-style-type: none"> ▪ Installation in dry and wet concrete (not flooded boreholes) ▪ Overlap joint with existing reinforcement in a building component ▪ Anchoring of the reinforcement at a slab or beam support ▪ Anchoring of reinforcement of building components stressed primarily in compression ▪ Anchoring of reinforcement to cover the envelope line of tensile force in the bending member
Material & Durability	<ul style="list-style-type: none"> ▪ Rebar Class B and C as EN 1992-1-1:2004+AC:2010, Annex C ▪ ZA Tension Anchor B500 B as DIN 488 for internal and external use without particular aggressive conditions ▪ ZA Tension Anchor Stainless Steel A4 as DIN 488 for internal and external use without particular aggressive conditions ▪ ZA Tension Anchor High Resistance Stainless Steel HCR as DIN 488 for all conditions
Loading	Static, quasi-static and Fire exposure as EN1992-1
Temperature Range	-40°C to +80°C max long term temperature +50°C and max short term temperature +80°C
Fire Reaction	A1 according to EN 13501-1

 4. Manufacturer (art. 11.5): **Friulsideer SpA via trieste,1 - 33048 San Giovanni al Natisone (UD) - Italy**

 5. Authorised representative (art. 12.2): **Not Relevant**

 6. System of Assessment AVCP (annex V): **System 1**

7/8. Harmonised Specification & Notified Body:

	Name of Body	System of Assessment	Reference	EAD / hEN Document
Technical Specification Document	DiBt ^[TAB]	1	ETA-16/0957	EAD 330499-01-0601
Constancy of Performance & FPC	MPA Darmstadt 1343 ^[NB]	1	1343-CPR-M 527-9	EAD 330499-01-0601
Technical Specification Document	DiBt ^[TAB]	1	ETA-16/0961	EAD 330087-00-0601
Constancy of Performance & FPC	MPA Darmstadt 1343 ^[NB]	1	1343-CPR-M 527-8	EAD 330087-00-0601

 9. Declared Performance: **See Annexes**

10. The performance of the product identified in points 1 and 2 is in conformity with declared performance in point 9.

This declaration of performance is issued under the sole responsibility of Friulsideer SpA.

Signed for and behalf of the manufacturer by:

Function	Name	Signature	Place and date of issue
Technical Manager	Raffaele Palmieri		San Giovanni al Natisone, 06-03-2020

ANNEX I°

Declared Performances acc. to ETA-16/0957 & EAD 330499-01-0601 - Design method acc. to EN 1992-4:2018

ESSENTIAL CHARACTERISTICS			PERFORMANCE - <u>THREADED RODS</u>								
Installation parameters			d	M8	M10	M12	M16	M20	M24	M27	M30
d₀	Nominal diameter of drill bit	[mm]	10	12	14	18	22	28	30	35	
h_{ef}	Effective embedment depth	h _{ef,min} [mm]	60	60	70	80	90	96	108	120	
		h _{ef,std} [mm]	80	90	110	125	170	210	240	270	
		h _{ef,max} [mm]	160	200	240	320	400	480	540	600	
h_{min}	Minimum thickness of the concrete member	[mm]	h _{ef} + 30 ≥ 100mm				h _{ef} + 2•d ₀				
T_{inst}	Torque moment (max)	[Nm]	10	20	40	60	100	170	250	300	
S_{min}	Minimum spacing	[mm]	40	50	60	75	95	115	125	140	
C_{min}	Minimum edge distance	[mm]	35	40	45	50	60	65	75	80	
TENSION Steel failure											
N_{Rk,s}	Tension Steel characteristic failure	cl. 4.8 - 4.6 [kN]	15	23	34	63	98	141	184	224	
		cl. 5.8 - 5.6 [kN]	18	29	42	78	122	176	230	280	
		cl. 8.8 [kN]	29	46	67	125	196	282	368	449	
		A4-70 (50) [kN]	26	41	59	110	171	247	(230)	(281)	
γ_{m,sN}¹⁾	Partial safety factor	cl. 4.6-5.6 [-]	2,0								
		cl. 4.8-5.8-8.8 [-]	1,5								
		A4-70 (50) [-]	1,87							(2,86)	
Combined pull-out and concrete failure			M8	M10	M12	M16	M20	M24	M27	M30	
τ_{Rk,ucr}	Characteristic bond resistance for un-cracked concrete C20/25	80°/50°C [MPa]	17	17	16	15	14	13	13	13	
		120°/72°C [Mpa]	15	14	14	13	12	12	11	11	
		160°/100°C [MPa]	12	11	11	10	9,5	9	9	9	
τ_{Rk,cr}	Characteristic bond resistance for cracked concrete C20/25 and Seismic Category C1	80°/50°C [MPa]	7,0	7,5	8	9	8,5	7	7	7	
		120°/72°C [Mpa]	6	6,5	7	7,5	7	6	6	6	
		160°/100°C [MPa]	5,5	5,5	6	6,5	6	5,5	5,5	5,5	
τ_{Rk,seisC2}	Characteristic bond resistance for Seismic Category C2	80°/50°C [MPa]	-	-	3,6	3,5	3,3	2,3	-	-	
		120°/72°C [Mpa]	-	-	3,1	3	2,8	2	-	-	
		160°/100°C [MPa]	-	-	2,5	2,7	2,5	1,8	-	-	
ψ_c	Increasing factor for concrete	C30/37 [-]	1,04								
		C40/50 [-]	1,08								
		C50/60 [-]	1,10								
ψ_c	Increasing factor for concrete for Seismic action	from C25/30 to C50/60 [-]	1,0								
ψ⁰_{sus}	Reduction factor for concrete C20/25 cracked, un-cracked and Seismic action	80°/50°C [-]	0,79								
		120°/72°C [-]	0,75								
		160°/100°C [-]	0,66								
Concrete cone failure											
K_{cr,N}	Factor acc. to EN 1992-4 § 7.2.1.4 cracked	[-]	7,7								
K_{ucr,N}	Factor acc. to EN 1992-4 § 7.2.1.4 un-cracked	[-]	11,0								
C_{cr,N}	Critical edge distance	[mm]	1,5 • h _{ef}								
S_{cr,N}	Critical spacing	[mm]	2,0 • C _{cr,N}								
Splitting failure											
C_{cr,sp}	Critical edge distance for Splitting	h / h _{ef} ≥ 2,0	1,0 • h _{ef}								
		2,0 > h / h _{ef} > 1,3	2,0 • h _{ef} • (2,5 - h / h _{ef})								
		h / h _{ef} ≤ 1,3	2,4 • h _{ef}								
S_{cr,sp}	Critical spacing for Splitting	[mm]	2,0 • C _{cr,sp}								
γ₂ = γ_{inst}	Installation safety factor	MAC ¹⁾ [-]	1,2								
		CAC (HDB) ¹⁾ [-]	1,0 (1,2)								
		CAC ¹⁾ for flooded borehole [-]	1,4								
Displacement under Tension Load in Concrete			M8	M10	M12	M16	M20	M24	M27	M30	
δ_{No}	Short term displacement un-cracked concrete	80°/50°C [mm/MPa]	0,031	0,032	0,034	0,037	0,039	0,042	0,044	0,046	
		120°/72°C [mm/MPa]	0,032	0,034	0,035	0,038	0,041	0,044	0,046	0,048	
		160°/100°C [mm/MPa]	0,121	0,126	0,131	0,142	0,153	0,163	0,171	0,179	
δ_{No∞}	Long term displacement un-cracked concrete	80°/50°C [mm/MPa]	0,040	0,042	0,044	0,047	0,051	0,054	0,057	0,060	
		120°/72°C [mm/MPa]	0,042	0,044	0,045	0,049	0,053	0,056	0,059	0,062	
		160°/100°C [mm/MPa]	0,124	0,129	0,135	0,146	0,157	0,168	0,176	0,184	
δ_{No}	Short term displacement cracked concrete and Seismic C1	80°/50°C [mm/MPa]	0,081	0,083	0,085	0,090	0,095	0,099	0,103	0,106	
		120°/72°C [mm/MPa]	0,084	0,086	0,088	0,093	0,098	0,103	0,107	0,110	
		160°/100°C [mm/MPa]	0,312	0,321	0,330	0,349	0,367	0,385	0,399	0,412	
δ_{No∞}	Long term displacement cracked concrete and Seismic C1	80°/50°C [mm/MPa]	0,104	0,107	0,110	0,116	0,122	0,128	0,133	0,137	
		120°/72°C [mm/MPa]	0,108	0,111	0,114	0,121	0,127	0,133	0,138	0,143	
		160°/100°C [mm/MPa]	0,321	0,330	0,340	0,358	0,377	0,396	0,410	0,424	
δ_{N,eq(DLS)}	Displacement for Seismic C2 at DLS	[mm]	-	-	0,24	0,27	0,29	0,27	-	-	
δ_{N,eq(ULS)}	Displacement for Seismic C2 at ULS	[mm]	-	-	0,55	0,51	0,50	0,58	-	-	

¹⁾ CAC = Cleaning with Compressed air, MAC = Cleaning with Manual Hand Pump and HDB = cleaning with Hollow drill bit system

ANNEX II°

Declared Performances acc. to ETA-16/0957 & ETAG001 p.1-5 - Design method acc. to EN 1992-4:2018

ESSENTIAL CHARACTERISTICS			PERFORMANCE - <u>THREADED RODS</u>							
SHEAR Steel failure			M8	M10	M12	M16	M20	M24	M27	M30
$V_{Rk,s}$	Shear Steel characteristic failure	cl. 4.8 - 4.6 [kN]	9	14	20	38	59	85	110	135
		cl. 5.8 - 5.6 [kN]	11	17	25	47	74	106	138	168
		cl. 8.8 [kN]	15	23	34	63	98	141	184	224
		A4-70 (50) [kN]	13	20	30	55	86	124	(115)	(140)
$V_{Rk,s,eq,C1}$	Shear Steel characteristic failure Seismic C1	[kN]	$0,70 \cdot V_{Rk,s}$							
$V_{Rk,s,eq,C2}$	Shear Steel characteristic failure Seismic C2	[kN]	-	-	$0,70 \cdot V_{Rk,s}$				-	-
$M_{Rk,s}^0$	Characteristic Bending Moment	cl. 4.8 - 4.6 [Nm]	15	30	52	133	260	449	666	900
		cl. 5.8 - 5.6 [Nm]	19	37	65	166	324	560	833	1123
		cl. 8.8 [Nm]	30	60	105	266	519	896	1333	1797
		A4-70 (50) [Nm]	26	52	92	232	454	784	(832)	(1125)
$M_{Rk,s,C1}^0$	Characteristic Bending Moment Seismic C1	[kN]	NPD [No Performance Data]							
$M_{Rk,s,C2}^0$	Characteristic Bending Moment Seismic C2	[kN]	NPD [No Performance Data]							
$\gamma_{m,sV}$	Partial safety factor	cl. 4.6-5.6 [-]	1,67							
		cl. 4.8-5.8-8.8 [-]	1,25							
		A4-70 (50) [-]	1,56						(2,38)	
k_7	Ductility factor acc. to EN 1992-4 § 7.2.2.3.1	[-]	1,0							
Concrete pry-out failure										
k_8	Factor acc. to EN 1992-4 § 7.2.2.4	[-]	2,0							
$\gamma_2 = \gamma_{inst}$	Installation safety factor	[-]	1,0							
Concrete edge failure			see CEN/TS 1992-4-5 Section 6.3.4							
l_f	Effective length of anchor	[-]	$l_f = \min(h_{ef}; 12 \cdot d_{nom})$						$l_f = \min(h_{ef}; 300mm)$	
d_{nom}	Outside diameter of anchor	[mm]	8	10	12	16	20	24	27	30
$\gamma_2 = \gamma_{inst}$	Installation safety factor	[-]	1,0							
Displacement under Shear Load			M8	M10	M12	M16	M20	M24	M27	M30
δ_{V0}	Short term displacement in Concrete and Seismic C1	[mm/kN]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
$\delta_{V\infty}$	Long term displacement in Concrete and Seismic C1	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05
$\delta_{V,eq}(DLS)$	Displacement for Seismic C2 at DLS ⁽¹⁾	[mm]	-	-	3,6	3,0	3,1	3,5	-	-
$\delta_{V,eq}(ULS)$	Displacement for Seismic C2 at ULS ⁽¹⁾	[mm]	-	-	7	6,6	7,0	9,3	-	-

(1)DLS= damage limitation state, ULS=ultimate limit state.

ANNEX III°

Declared Performances acc. to ETA-16/0957 & EAD 330499-01-0601 - Design method acc. to EN 1992-4:2018

ESSENTIAL CHARACTERISTICS				PERFORMANCE - REBAR									
Installation parameters				d	Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
d₀	Nominal diameter of drill bit	[mm]		12	14	16	18	20	25	32	35	40	
h_{ef}	Effective embedment depth	h_{ef,min}	[mm]	60	60	70	75	80	90	100	112	128	
		h_{ef,std}	[mm]	80	90	110	115	125	170	210	250	280	
		h_{ef,max}	[mm]	160	200	240	280	320	400	500	560	640	
h_{min}	Minimum thickness of the concrete member	[mm]	$h_{ef} + 30 \geq 100$				$h_{ef} + 2 \cdot d_0$						
s_{min}	Minimum spacing	[mm]	40	50	60	70	75	95	120	130	150		
c_{min}	Minimum edge distance	[mm]	35	40	45	50	50	60	70	75	85		
TENSION Steel failure													
N_{Rk,s}	Tension Steel characteristic failure	[kN]	$A_s \cdot f_{uk}^{2)}$										
A_s	Area resistant	[mm ²]	50	79	113	154	201	314	491	616	804		
γ_{m,sN}	Partial safety factor	[-]	1,4 ³⁾										
Combined pull-out and concrete cone failure				Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
τ_{Rk,ucr}	Characteristic bond resistance in un-cracked concrete C20/25	80°/50°C	[MPa]	14	14	14	14	13	13	13	13	13	
		120°/72°C	[MPa]	13	12	12	12	12	11	11	11	11	
		160°/100°C	[MPa]	9,5	9,5	9,5	9	9	9	9	8,5	8,5	
τ_{Rk,cr}	Characteristic bond resistance in cracked concrete C20/25	80°/50°C	[MPa]	5,5	5,5	6	6	7,5	7,5	7,5	7,5	8	
		120°/72°C	[MPa]	4,5	5	5	5,5	6,5	6,5	6,5	6,5	7	
		160°/100°C	[MPa]	4	4,5	4,5	5	5,5	6	6	5,5	6,5	
ψ_c	Increasing factor for concrete	C30/37	[-]	1,04									
		C40/50	[-]	1,08									
		C50/60	[-]	1,10									
ψ_c	Increasing factor for concrete for Seismic action	da C25/30 a C50/60	[-]	1,0									
ψ_{sus}⁰	Reduction factor for concrete C20/25 cracked, un-cracked and Seismic action	80°/50°C	[-]	0,79									
		120°/72°C	[-]	0,75									
		160°/100°C	[-]	0,66									
Concrete cone failure													
k_{cr,N}	Factor acc. to EN 1992-4 § 7.2.1.4 cracked	[-]	7,7										
k_{ucr,N}	Factor acc. to EN 1992-4 § 7.2.1.4 un-cracked	[-]	11										
c_{cr,N}	Critical edge distance	[mm]	$1,5 \cdot h_{ef}$										
s_{cr,N}	Critical spacing	[mm]	$2,0 \cdot c_{cr,N}$										
Splitting failure				Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
c_{cr,sp}	Critical edge distance for Splitting	$h / h_{ef} \geq 2,0$	[mm]	$1,0 \cdot h_{ef}$									
		$2,0 > h / h_{ef} > 1,3$	[mm]	$2,0 \cdot h_{ef} \cdot (2,5 - h / h_{ef})$									
		$h / h_{ef} \leq 1,3$	[mm]	$2,4 \cdot h_{ef}$									
s_{cr,sp}	Critical spacing for Splitting	[mm]	$2,0 \cdot c_{cr,sp}$										
γ₂ = γ_{inst}	Installation safety factor	MAC ¹⁾	[-]	1,2				-					
		CAC (HDB) ¹⁾	[-]	1,0 (1,2)									
		CAC ¹⁾ for flooded borehole	[-]	1,4									
Displacement under Tension Load				Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
δ_{N0}	Short term displacement un-cracked concrete	80°/50°C	[mm/MPa]	0,031	0,032	0,034	0,035	0,037	0,039	0,043	0,045	0,048	
		120°/72°C	[mm/MPa]	0,032	0,034	0,035	0,036	0,038	0,041	0,045	0,047	0,050	
		160°/100°C	[mm/MPa]	0,121	0,126	0,131	0,137	0,142	0,153	0,164	0,172	0,186	
δ_{N∞}	Long term displacement un-cracked concrete	80°/50°C	[mm/MPa]	0,040	0,042	0,044	0,045	0,047	0,051	0,055	0,058	0,063	
		120°/72°C	[mm/MPa]	0,042	0,044	0,045	0,047	0,049	0,053	0,057	0,060	0,065	
		160°/100°C	[mm/MPa]	0,124	0,129	0,135	0,141	0,146	0,157	0,169	0,177	0,192	
δ_{N0}	Short term displacement cracked concrete and Seismic C1	80°/50°C	[mm/MPa]	0,081	0,083	0,085	0,087	0,090	0,095	0,099	0,103	0,108	
		120°/72°C	[mm/MPa]	0,084	0,086	0,088	0,090	0,093	0,098	0,103	0,107	0,113	
		160°/100°C	[mm/MPa]	0,312	0,321	0,330	0,340	0,349	0,367	0,385	0,399	0,425	
δ_{N∞}	Long term displacement cracked concrete and Seismic C1	80°/50°C	[mm/MPa]	0,104	0,107	0,110	0,113	0,116	0,122	0,128	0,133	0,141	
		120°/72°C	[mm/MPa]	0,108	0,111	0,114	0,118	0,121	0,127	0,133	0,138	0,148	
		160°/100°C	[mm/MPa]	0,321	0,330	0,340	0,349	0,358	0,377	0,396	0,410	0,449	

¹⁾ CAC = Cleaning with Compressed air, MAC = Cleaning with Manual Hand Pump and HDB = cleaning with Hollow drill bit system

²⁾ f_{uk} shall be taken from the specifications of reinforcing bars

³⁾ In absence of other national regulations.

⁴⁾ Calculation of the displacement = $\delta_{N \tau}$ ($\tau = \tau_{rk} / \gamma_2 \times 1,5 \times 1,4$)

ANNEX IV°

Declared Performances acc. to ETA-16/0957 & 330499-01-0601 - Design method acc. to EN 1992-4:2018

ESSENTIAL CHARACTERISTICS		PERFORMANCE - REBAR									
SHEAR Steel failure		Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
$V_{Rk,s}$	Shear Steel characteristic failure [kN]	0,5 · $N_{Rk,s}$									
$V_{Rk,s,eq,C1}$	Shear Steel characteristic failure Seismic C1 [kN]	0,37 · $N_{Rk,s}$									
$M_{Rk,s}^0$	Bending Moment characteristic failure [Nm]	1,2 · $W_{el} \cdot f_{uk}^{1)}$									
$M_{Rk,s,eq,C1}^0$	Bending Moment characteristic failure Seismic C1 [Nm]	NPD [No Performance Data]									
W_{el}	Elastic section modulus [mm ³]	50	98	170	269	402	785	1534	2155	3217	
$\gamma_{m,sV}$	Partial safety factor [-]	1,5 ²⁾									
k_7	Ductility factor acc. to EN 1992-4 § 7.2.2.3.1 [-]	0,8									
Concrete Pryout failure											
k_8	Factor acc. to EN 1992-4 § 7.2.2.4 [-]	2,0									
$\gamma_2 = \gamma_{inst}$	Installation safety factor [-]	1,0									
Concrete Edge failure											
l_f	Effective length of anchor [-]	min (h_{ef} ; 12 · d_{nom})						min (h_{ef} ; 300mm)			
d_{nom}	Outside diameter of anchor [mm]	8	10	12	14	16	20	25	28	32	
$\gamma_2 = \gamma_{inst}$	Installation safety factor [-]	1,0									
Displacement under Shear Load ³⁾		Ø8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32	
δ_{V0}	Short term displacement in Concrete and Seismic C1 [mm/kN]	0,06	0,05	0,05	0,04	0,04	0,04	0,03	0,03	0,03	
$\delta_{V\infty}$	Long term displacement in Concrete and Seismic C1 [mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,04	0,04	

¹⁾ f_{uk} shall be taken from the specifications of reinforcing bars

²⁾ In absence of other national regulations

³⁾ Calculation of the displacement = $\delta_V \times V$ ($V = V_{Rk,s} / \gamma_2 \times 1,5 \times 1,4$)

ANNEX V°

Declared Performances acc. to ETA-16/0961 & EAD 330087-00-0601															
Design method acc. to EN 1992-1-1:2004+AC:2010 and ETA-16/0961															
ESSENTIAL CHARACTERISTICS				PERFORMANCE POST-INSTALLED REBAR CONNECTION											
Installation parameters				d	Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø22	Ø24	Ø25	Ø28	Ø32
d₀	Nominal diameter of drill bit			[mm]	12	14	16	18	20	25	28	32	32	35	40
ℓ_{v,MAX}	Maximum embedment depth			[mm]	see table B6 of ETA-16/0961										
ℓ_{b,min}	Minimum anchorage length			[mm]	§ 8.6 - § 8.7 EN 1992-1-1:2004+AC2010										
ℓ_{0,min}	Lap length			[mm]	§ 8.11 EN 1992-1-1:2004+AC2010										
α_{lb}	Amplification factor for ℓ _{b,min} and ℓ _{0,min}			[-]	1,0										
c ¹⁾²⁾	Minimum concrete cover min c	Without	hammer drilling HD	[mm]	30 mm + 0,06·ℓ _v ≥ 2·Ø						40 mm + 0,06·ℓ _v ≥ 2·Ø				
		drilling Aid	compr. air drilling CD	[mm]	50 mm + 0,08·ℓ _v						60 mm + 0,08·ℓ _v				
		With	hammer drilling HD	[mm]	30 mm + 0,02·ℓ _v ≥ 2·Ø						40 mm + 0,02·ℓ _v ≥ 2·Ø				
		drilling Aid	compr. air drilling CD	[mm]	50 mm + 0,02·ℓ _v						60 mm + 0,02·ℓ _v				
s_{min}	Minimum spacing			[mm]	≥ 5·Ø ≥ 50 mm										
Design values of ultimate bond resistance															
f_{bd}	Bond design value resistance "for all drilling methods for good conditions"	C12/15	[N/mm ²]		1,6										
		C16/20	[N/mm ²]		2,0										
		C20/25	[N/mm ²]		2,3										
		C25/30	[N/mm ²]		2,7										
		C30/37	[N/mm ²]		3,0										
		C35/45	[N/mm ²]		3,4										
		C40/50	[N/mm ²]		3,7										
		C45/55	[N/mm ²]		4,0										
	C50/60	[N/mm ²]		4,3											
f_{bd,c}	"for all other bond conditions"			[N/mm ²]	f_{bd} · 0,7										
FIRE EXPOSURE Design method acc. to EN 1992-1-1:2004+AC:2008															
f_{bd,fi}	Bond design value resistance Under FIRE EXPOSURE			[N/mm ²]	³⁾ f_{bd,fi} = k_{b,fi}(θ) · f_{bd} · γ_c / γ_{M,fi}										

- 1) Not allowed Diamond Drilling
- 2) The minimum concrete cover acc. EC 1992-1-1:2004+AC:2010 must be observed
- 3) With: $k_{b,fi}(\theta)$ = reduction factor under fire exposure (see graphics below)
 - f_{bd} = see table above
 - γ_c = partial safety factor acc.to EN 1992-1-1
 - $\gamma_{M,fi}$ = partial safety factor acc.to EN 1992-1-2 under fire exposure

Example graph of Reduction factor $k_{b,fi}(\theta)$ for concrete classes C20/25 for good bond conditions:

