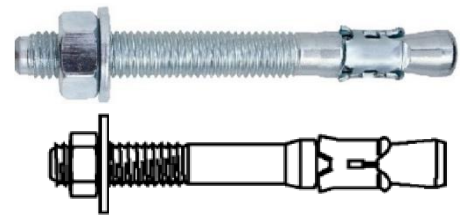


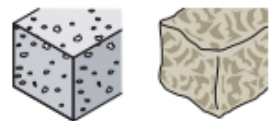
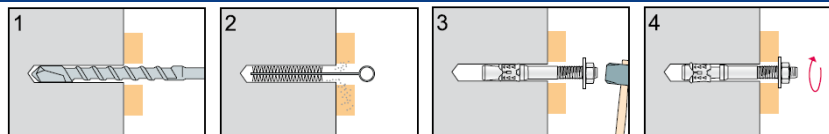
## FM753 FM753 CRACK OPTION 1



### PRODUCT DESCRIPTION

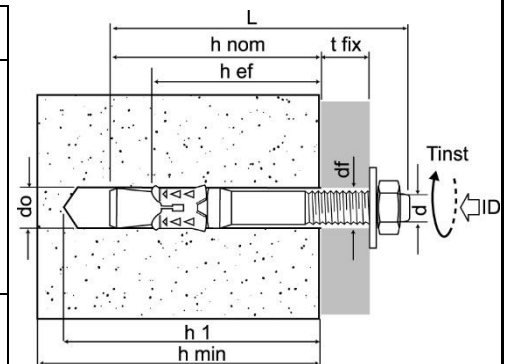
Heavy duty through anchor for cracked and un-cracked concrete.

### APPLICATION



### LENGTH OF SCREWS

d	size d x L	ID	tfix [mm]	do [mm]	h1 [mm]	hnom [mm]	hef [mm]	df [mm]	hmin [mm]	Tinst [Nm]	Sw
M8	M8x68	A	4	8	70	54	48	9	100	20	13
	M8x75	B	10								
	M8x90	C	25								
	M8x115	D	50								
	M8x135	E	70								
	M8x165	G	100								
M10	M10x90	A	10	10	80	67	60	12	120	40	17
	M10x105	B	25								
	M10x115	C	35								
	M10x135	D	55								
	M10x155	E	75								
	M10x185	F	105								
M12	M12x110	A	10	12	100	81	72	14	150	60	19
	M12x120	B	20								
	M12x145	C	45								
	M12x170	D	70								
	M12x200	E	100								
M16	M16x130	A	10	16	115	97	86	18	170	120	24
	M16x150	B	30								
	M16x185	C	60								
	M16x220	D	100								



- tfix = fixture thickness
- do = hole diameter
- h1 = minimum hole depth
- hnom = nominal embedment depth
- hef = minimum depth of anchorage
- df = hole diameter of fixing element
- hmin = minimum support thickness
- Tinst = torque
- d = screw diameter
- L = anchor length
- sw = wrench
- ID = ident. mark, product length

### Anchor body characteristics

Anchor diameter		M8	M10	M12	M16
Tensile stressed cross-section	$A_{s,N}$ [mm <sup>2</sup> ]	26,4	43,0	60,8	109,4
Shear stressed cross-section	$A_{s,V}$ [mm <sup>2</sup> ]	36,6	58	84,3	157
Bending moment - Galvanized steel anchor body grade 9.8	M [Nm]	16	32	56	143
Bending moment - Stainless steel A4 anchor body	M [Nm]	13	27	47	120

## EUROPEAN TECHNICAL APPROVAL ETA-09/0056

### FM-753 CRACK 3DG DESIGN<sup>1)</sup> and RECOMMENDED<sup>2)</sup> LOADS

Single anchor with large anchor spacing and edge distances in concrete C20/25

Anchor diameter		M8	M10	M12	M16
Depth of anchorage	$h_{ef}$ [mm]	48	60	72	86
Tensile - cracked concrete	$N_{rd,cr}^{(1)}$ [kN]	4,0	8,0	10,7	13,3
	$N_{cr}^{(2)}$ [kN]	2,9	5,7	7,6	9,5
Tensile - uncracked concrete	$N_{rd,ucr}^{(1)}$ [kN]	6,0	10,7	13,3	23,3
	$N_{ucr}^{(2)}$ [kN]	4,3	7,6	9,5	16,7
Increasing factor for tensile load	concrete C30/37 $\psi_{C,c,C30/37}$ [-]	1,22			
	concrete C40/50 $\psi_{C,c,C40/50}$ [-]	1,41			
	concrete C50/60 $\psi_{C,c,C50/60}$ [-]	1,55			
Spacing	$S_{cr,N}$ [mm]	140	180	220	260
Edge distance	$C_{cr,N}$ [mm]	70	90	110	130
Shear <sup>(3)</sup> $C \geq 10xh_{ef}$	$V_{rd}^{(1)}$ [kN]	8,6	16,1	22,5	44,3
	$V^{(2)}$ [kN]	6,1	11,5	16,1	31,6

1kN = 100 kgf

<sup>(1)</sup> The design loads  $N_{rd}$  and  $V_{rd}$  derive from the characteristic loads on the ETA-09/0056 certification and are inclusive of the partial safety factors  $\gamma_m$  proportional to each diameter (see ETA).

<sup>(2)</sup> The recommended loads  $N$  and  $V$  derive from the characteristic loads on the ETA-09/0056 certification and are inclusive of the partial safety factors  $\gamma_{T1}=1,4$  and  $\gamma_m$  proportional to each diameter (see ETA).

<sup>(3)</sup> Shear values valid with distance from the edge  $C \geq 10xh_{ef}$ .

### Minimum installation distances

Anchor diameter		M8	M10	M12	M16
Minimum distance from edge	$C_{min}$ [mm]	50	60	70	85
	per / for $S \geq$ [mm]	75	120	150	170
Minimum distance between anchors	$S_{min}$ [mm]	50	60	70	80
	per / for $C \geq$ [mm]	65	80	90	120

Example (according to annex C of the ETAG 001) of shear load across the C20/25 concrete edge at a distance of  $C_{min}$

Anchor diameter		M8	M10	M12	M16
Shear $C = C_{min}$ cracked concrete	$V_{rd,cmin}$ [kN]	3,2	4,4	5,8	8,1
	$V_{cmin}$ [kN]	2,3	3,2	4,1	5,8

## FIRE RESISTANCE

### Characteristic Fire Resistance (all direction)

Anchor diameter		M8	M10	M12	M16
Characteristic Fire Resistance = 30 min.	$F_{Rk,s,fi,30}$ [kN]	0,4	0,9	1,7	3,1
Characteristic Fire Resistance = 60 min.	$F_{Rk,s,fi,60}$ [kN]	0,3	0,8	1,3	2,4
Characteristic Fire Resistance = 90 min.	$F_{Rk,s,fi,90}$ [kN]	0,3	0,6	1,1	2,0
Characteristic Fire Resistance = 120 min.	$F_{Rk,s,fi,120}$ [kN]	0,2	0,5	0,8	1,6
Spacing	$S_{cr,N,fi}$ [mm]	192	240	288	384
Edge distance	$C_{cr,N,fi}$ [mm]	96	120	144	192

## SEISMIC RESISTANCE

### Seismic Resistance for Category C1

Anchor diameter		M8	M10	M12	M16
Tensile in concrete for C20/25 Seismic Category C1	$N_{rd,seis C1}^{(1)}$ [kN]	4,0	8,0	10,7	13,3
	$N_{seis C1}^{(2)}$ [kN]	2,9	5,7	7,6	9,5
Shear for Seismic Category C1	$V_{rd,s,seis C1}^{(1)}$ [kN]	5,1	11,3	20,3	38,4
	$V_{s,seis C1}^{(2)}$ [kN]	3,7	8,1	14,5	27,4

### Seismic Resistance for Category C2

Anchor diameter		M8	M10	M12	M16
Tensile in concrete for C20/25 Seismic Category C1	$N_{rd,seis C1}^{(1)}$ [kN]	-	2,2	7,9	13,3
	$N_{seis C1}^{(2)}$ [kN]	-	1,6	5,6	9,5
Shear for Seismic Category C1	$V_{rd,s,seis C1}^{(1)}$ [kN]	-	7,9	12,9	20,8
	$V_{s,seis C1}^{(2)}$ [kN]	-	5,7	9,2	14,9

<sup>(1)</sup>  $N_{rd,seis}$  e  $V_{rd,seis}$  = Design loads under seismic action (included  $\gamma_m=1,5$  see ETA)

<sup>(2)</sup>  $N_{seis}$  e  $V_{seis}$  = Recommended loads under seismic action (included  $\gamma_m \gamma_f = 1,5 \times 1,4$  see ETA)

### CALCULATION OF DESIGN SEISMIC RESISTANCE

**Tensile load**  $N_{d,seis} = \alpha_{gap} \cdot \alpha_{seis} \cdot N_{rd,seis}^0$

**Shear load**  $V_{d,seis} = \alpha_{gap} \cdot \alpha_{seis} \cdot V_{rd,seis}^0$

$\alpha_{gap}$  -  $\alpha_{se}$  = see reduction factors on the table under;

$N_{rd,seis}^0$  -  $V_{rd,seis}^0$  = lowest value among the tables above and other failure modes see ETA-09/0056 and CEN/TS 1992-4.

Reduction factors for resistance under seismic actions		Tension failure					Shear failure		
		Steel	Pull-out	Comb.	Concr. cone	Splitting	Steel	Concr. Edge	Pry-out
		[N <sub>Rk,s</sub> ]	[N <sub>Rk,p</sub> ]	[N <sub>Rk,p-c</sub> ]	[N <sub>Rk,c</sub> ]	[N <sub>Rk,sp</sub> ]	[V <sub>Rk,s</sub> ]	[V <sub>Rk,c</sub> ]	[V <sub>Rk,cp</sub> ]
$\alpha_{gap}$	Reduction factor for gap hole fixture and fasteners	1,00	1,00	1,00	1,00	1,00	0,50*	0,50*	0,50*
$\alpha_{seis}$	Reduction factor for single fasteners	1,00	1,00	1,00	0,85	1,00	1,00	1,00	0,85
	Reduction factor for fasteners group	1,00	0,85	0,85	0,75	0,85	0,85	0,85	0,75

\*  $\alpha_{gap} = 1,0$  in case of no clearance between fastener and fixture.

Recommended seismic performance categories for anchors					
Seismicity level <sup>a)</sup>		Importance Class acc. to EN 1998-1:2004, 4.2.5			
	$a_g \cdot S$ <sup>c)</sup>	I	II	III	IV
Very low <sup>b)</sup>	$a_g \cdot S \leq 0,05 \text{ g}$	No additional requirement			
Low <sup>b)</sup>	$0,05 \text{ g} < a_g \cdot S \leq 0,1 \text{ g}$	C1	C1 <sup>d)</sup> or C2 <sup>e)</sup>		C2
> Low <sup>b)</sup>	$a_g \cdot S > 0,1 \text{ g}$	C1	C2		

<sup>a)</sup> The values defining the seismicity levels may be found in the National Annex of EN 1998-1 (Eurocode 8)  
<sup>b)</sup> Definition according to EN 1998-1:2004, 3.2.1.  
<sup>c)</sup>  $a_g$  = Design ground acceleration on type A ground (EN 1998-1:2004, Table 3.2.1)  
 $S$  = Soil factor (see e.g. EN 1998-1:2004, 3.2.2)  
<sup>d)</sup> C1 for fixing non-structural elements to structure  
<sup>e)</sup> C2 for fixing structural elements to structure

For additional information see ETA certifications.

The load values are only valid if the installation has been carried out correctly. The design engineer is responsible for the designing and calculation of the fixing.

## EUROPEAN TECHNICAL APPROVAL ETA-10/0293

### FM-753 CRACK A4 DESIGN<sup>(1)</sup> and RECOMMENDED<sup>(2)</sup> LOADS

Single anchor with large anchor spacing and edge distances in concrete C20/25

Anchor diameter		M8	M10	M12	M16
Depth of anchorage	$h_{ef}$ [mm]	48	60	72	86
Tensile - cracked concrete	$N_{rd,cr}^{(1)}$ [kN]	3,3	6,0	8,0	16,7
	$N_{cr}^{(2)}$ [kN]	2,4	4,3	5,7	11,9
Tensile - un-cracked concrete	$N_{rd,ucr}^{(1)}$ [kN]	6,0	10,7	13,3	23,3
	$N_{ucr}^{(2)}$ [kN]	4,3	7,6	9,5	16,7
Increasing factor for tensile load	concrete C30/37 $\psi_{C,c,C30/37}$ [-]	1,22			
	concrete C40/50 $\psi_{C,c,C40/50}$ [-]	1,41			
	concrete C50/60 $\psi_{C,c,C50/60}$ [-]	1,55			
Spacing	$S_{cr,N}$ [mm]	144	180	216	258
Edge distance	$C_{cr,N}$ [mm]	72	90	108	129
Shear <sup>(3)</sup> $C \geq 10xh_{ef}$	$V_{rd}^{(1)}$ [kN]	9,1	14,5	21,1	39,2
	$V^{(2)}$ [kN]	6,5	10,4	15,1	28,0

1kN = 100 kgf

<sup>(1)</sup> The design loads  $N_{rd}$  and  $V_{rd}$  derive from the characteristic loads on the ETA-10/0293 certification and are inclusive of the partial safety factors  $\gamma_m$  proportional to each diameter (see ETA).

<sup>(2)</sup> The recommended loads  $N$  and  $V$  derive from the characteristic loads on the ETA-10/0293 certification and are inclusive of the partial safety factors  $\gamma_{ft}=1,4$  and  $\gamma_{fm}$  proportional to each diameter (see ETA).

<sup>(3)</sup> Shear values valid with distance from the edge  $C \geq 10xh_{ef}$ .

#### Minimum installation distances

Anchor diameter		M8	M10	M12	M16
Minimum distance from edge	$C_{min}$ [mm]	50	50	60	70
	per / for $S \geq$ [mm]	50	110	120	130
Minimum distance between anchors	$S_{min}$ [mm]	50	55	60	70
	per / for $C \geq$ [mm]	50	70	80	100

Example (according to annex C of the ETAG 001) of shear load across the C20/25 concrete edge at a distance of  $C_{min}$

Tipo ancorante / Anchor diameter		M8	M10	M12	M16
Shear $C = C_{min}$ cracked concrete	$V_{rd,cmin}$ [kN]	3,2	3,5	4,7	6,3
	$V_{cmin}$ [kN]	2,3	2,5	3,4	4,5

## FIRE RESISTANCE

### Characteristic Fire Resistance (all direction)

Anchor diameter		M8	M10	M12	M16
Characteristic Fire Resistance = 30 min.	$F_{Rk,s,fi,30}$ [kN]	0,5	1,1	1,8	3,3
Characteristic Fire Resistance = 60 min.	$F_{Rk,s,fi,60}$ [kN]	0,4	0,9	1,5	2,7
Characteristic Fire Resistance = 90 min.	$F_{Rk,s,fi,90}$ [kN]	0,3	0,7	1,2	2,2
Characteristic Fire Resistance = 120 min.	$F_{Rk,s,fi,120}$ [kN]	0,3	0,6	1,0	1,8
Spacing	$S_{cr,N,fi}$ [mm]	192	240	288	384
Edge distance	$C_{cr,N,fi}$ [mm]	96	120	144	192

## SEISMIC RESISTANCE

### Seismic Resistance for Category C1

Anchor diameter		M8	M10	M12	M16
Tensile in concrete for C20/25 Seismic Category C1	$N_{rd,seis C1}^{(1)}$ [kN]	2,7	6,0	8,0	16,7
	$N_{seis C1}^{(2)}$ [kN]	2,0	4,3	5,7	11,9
Shear for Seismic Category C1	$V_{rd,s,seis C1}^{(1)}$ [kN]	6,2	9,5	12,2	28,2
	$V_{s,seis C1}^{(2)}$ [kN]	4,4	6,8	8,7	20,1

### Seismic Resistance for Category C2

Anchor diameter		M8	M10	M12	M16
Tensile in concrete for C20/25 Seismic Category C1	$N_{rd,seis C1}^{(1)}$ [kN]	-	1,6	5,9	14,6
	$N_{seis C1}^{(2)}$ [kN]	-	1,1	4,2	10,4
Shear for Seismic Category C1	$V_{rd,s,seis C1}^{(1)}$ [kN]	-	9,5	12,2	28,2
	$V_{s,seis C1}^{(2)}$ [kN]	-	6,8	8,7	20,1

<sup>(1)</sup> Design loads under seismic action (included  $\beta_m=1,5$  see ETA)

<sup>(2)</sup> Recommended loads under seismic action (included  $\beta_m \cdot \beta_f = 1,5 \times 1,4$  see ETA)

### CALCULATION OF DESIGN SEISMIC RESISTANCE

**Tensile load**  $N_{d,seis} = \alpha_{gap} \cdot \alpha_{seis} \cdot N_{rd,seis}^0$

**Shear load**  $V_{d,seis} = \alpha_{gap} \cdot \alpha_{seis} \cdot V_{rd,seis}^0$

$\alpha_{gap} - \alpha_{se}$  = see reduction factors on the table under;

$N_{rd,seis}^0 - V_{rd,seis}^0$  = lowest value among the tables above and other failure modes see ETA-09/0056 and CEN/TS 1992-4.

Reduction factors for resistance under seismic actions		Tension failure					Shear failure		
		Steel	Pull-out	Comb.	Concr. cone	Splitting	Steel	Concr. Edge	Pry-out
		[N <sub>Rk,s</sub> ]	[N <sub>Rk,p</sub> ]	[N <sub>Rk,p-c</sub> ]	[N <sub>Rk,c</sub> ]	[N <sub>Rk,sp</sub> ]	[V <sub>Rk,s</sub> ]	[V <sub>Rk,c</sub> ]	[V <sub>Rk,cp</sub> ]
$\alpha_{gap}$	Reduction factor for gap hole fixture and fasteners	1,00	1,00	1,00	1,00	1,00	0,50*	0,50*	0,50*
$\alpha_{seis}$	Reduction factor for single fasteners	1,00	1,00	1,00	0,85	1,00	1,00	1,00	0,85
	Reduction factor for fasteners group	1,00	0,85	0,85	0,75	0,85	0,85	0,85	0,75

\*  $\alpha_{gap} = 1,0$  in case of no clearance between fastener and fixture.

Recommended seismic performance categories for anchors					
Seismicity level <sup>a)</sup>		Importance Class acc. to EN 1998-1:2004, 4.2.5			
	$a_g \cdot S$ <sup>c)</sup>	I	II	III	IV
Very low <sup>b)</sup>	$a_g \cdot S \leq 0,05 \text{ g}$	No additional requirement			
Low <sup>b)</sup>	$0,05 \text{ g} < a_g \cdot S \leq 0,1 \text{ g}$	C1	C1 <sup>d)</sup> or C2 <sup>e)</sup>		C2
> Low <sup>b)</sup>	$a_g \cdot S > 0,1 \text{ g}$	C1	C2		

<sup>a)</sup> The values defining the seismicity levels may be found in the National Annex of EN 1998-1 (Eurocode 8)  
<sup>b)</sup> Definizione secondo EN 1998-1:2004, 3.2.1 / Definition according to EN 1998-1:2004, 3.2.1.  
<sup>c)</sup>  $a_g$  = Design ground acceleration on type A ground (EN 1998-1:2004, Table 3.2.1)  
 $S$  = Soil factor (see e.g. EN 1998-1:2004, 3.2.2)  
<sup>d)</sup> C1 for fixing non-structural elements to structure  
<sup>e)</sup> C2 for fixing structural elements to structure

For additional information see ETA certifications.

The load values are only valid if the installation has been carried out correctly. The design engineer is responsible for the designing and calculation of the fixing.

## OTHER FEATURES

<b>Type</b>	<b>3DG Brilliant *</b>	<b>Stainless steel A4</b>
<b>Anchor body</b>	Steel grade 9.8	Stainless steel AISI 316 (A4 70)
<b>Clip</b>	Stainless steel AISI 316 (A4)	
<b>Hex nut</b>	DIN 934 grade 8	DIN 934 A4 80
<b>Washer</b>	DIN 125/1	DIN 125/1 A4
<b>Coating*</b>	10 µm ISO 4042 (white-brillant)	-

\*Resistance salt spray test 1000 hours ISO 9227 (see technical data sheet of coating)

